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Suffusion susceptibility investigation by energy based method and statistical analysis

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ABSTRACT: Internal erosion is one of the main causes of instabilities within hydraulic earth structures. Four internal erosion processes can be distinguished and this study deals with the process of suffusion which corresponds to the coupled processes of detachment-transport-filtration of the soil's fine fraction between the coarse fraction. Because of the great length of earth structures and the heterogeneities of soils, it is very difficult to characterize the suffusion susceptibility of the different soils. Nevertheless, a statistical analysis can be performed in order to optimize the experimental campaign. By using a dedicated erodimeter, an experimental program was set up to study suffusion susceptibility of 31 specimens of non-plastic or low plasticity soils. The suffusion susceptibility is determined by the erosion resistance index, which relates the total loss mass with the total energy expended by the seepage flow. Fourteen physical parameters are selected and a multivariate statistical analysis leads to a correlation between the erosion resistance index and all these parameters. A statistical analysis is performed in order to identify the main parameters and to focus on those which can easily be measured on existing structures. By distinguishing gap-graded and widely-graded soils, two correlations are proposed to estimate the erosion resistance index.

***Key words*:** laboratory testing, suffusion, physical parameter, statistical analysis, energy.

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Introduction

Internal erosion is one of the main causes of instabilities within hydraulic earth structures such as dams, dikes or levees (Foster et al. 2000). According to Fell and Fry (2013), there are four types of internal erosion: concentrated leak erosion, backward erosion, contact erosion and suffusion. Concentrated leak erosion may occur through a crack or hydraulic fracture in cohesive soils. Backward erosion mobilizes all the grains in regressive way (i.e. from the downstream part of earth structure to the upstream part) and includes backward erosion piping and global backward erosion. Contact erosion occurs where a coarse soil is in contact with a fine soil. The phenomenon of suffusion corresponds to the process of detachment and then transport of the finest particles within the porous network of cohesionless soils. The nature of the soil in the earth structure and the boundary conditions (i.e. the presence or not of a downstream filter, of cracks or interfaces with other types of soils) determine soil vulnerability to each internal erosion process. Thus the soil erodibility must be identified taking into account all four internal erosion processes.

For the first three aforementioned processes of internal erosion, different classifications exist in order to evaluate the soil erodibility, whereas in the case of suffusion, only one susceptibility classification is available and has been recently proposed by Marot et al. (2016). The absence of several suffusion susceptibility classifications may be due to the complexity of this process, which appears as the result of the coupled processes: detachment – transport – filtration of a part of the finest fraction within the porous network. For this classification, the cumulative energy expended by the seepage flow is computed and the induced erosion is evaluated by the cumulative loss dry mass. Six categories of soil erodibility are proposed from very resistant to very erodible.

Soils of hydraulic earth structures and their foundations are characterized by great heterogeneities (for example: 8000 km of dikes in France and 13200 km of dikes in Vietnam). Due to this spatial variability

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of soils, the soil erodibility characterization requires a large number of erosion tests. In consequence, a statistical analysis is conducted for assessing the relationship between suffusion susceptibility and other properties of soils. By focusing on easily measurable parameters, this study contributes to an experimental campaign optimization.

For covering a large range of soil erodibility, eighteen non-plastic or low plasticity soils are selected and a total of 31 specimens are prepared. By using a dedicated erodimeter, the erodibility is evaluated for all specimens and test results are interpreted by the energy method. The suffusion susceptibility of all tested specimens is evaluated thanks to the erosion resistance index. Two tests are performed under identical conditions in order to verify the repeatability. The bibliographic study permits to identify fourteen predominant physical parameters which influence the soil suffusion susceptibility and which can be measured for soils of existing earth structures. A Principal Component Analysis is performed in order to determine the linear correlations between the erosion resistance index and these physical parameters. By eliminating the variables which are correlated or seem meaningless due to their redundant information with other variables on one hand, and by focusing on easily measurable parameters on the other hand, a new multivariable analysis allows to build up a correlation with a reduced number of physical parameters.

Background regarding experimental studies on suffusion

Identification of predominant parameters

Garner and Fannin (2010) describe the main initiation conditions for suffusion with the aid of a diagram comprising three components: material susceptibility, critical stress condition and critical hydraulic load. In the same manner, Fell and Fry (2013) highlight three criteria which have to be satisfied for suffusion to occur: geometric criterion, stress criterion and hydraulic criterion. The first

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two criteria are associated with the fabric of granular soils, and for non-plastic or low plasticity soils, soil fabric mainly depends on the grain size distribution, the particle shape and soil density. The soils that are likely to suffer from suffusion are, according to Fell and Fry (2007) “internally unstable”, i.e. their grain-size distribution curve is either discontinuous or upwardly concave. Based on this information, several criteria have been proposed in literature. In 1953, US Army Corps of Engineers, proposed to define the stability boundary by the uniformity coefficient C_u equal to 20. One of the most widely used criterion in literature to assess the internal stability of granular soils is the Kenney and Lau (1985)'s criterion. This criterion is based on the ratio H/F, where H is the mass fraction of a grain size distribution ranging from a diameter d to 4d and F is the mass fraction of particles finer than d. If the minimum value of this ratio, min (H/F) is smaller than 1.3 (revised to 1.0 by Kenney and Lau 1986), then the soil is classified as internally unstable.

For the interpretation of 20 suffusion tests, Wan and Fell (2008) used three criteria for predicting the initiation of suffusion. They concluded that these methods, based on particle size distribution are conservative and they proposed a method for assessing internal instability of broadly graded silt-sand-gravel soils. This method is based on two ratios: d_{90}/d_{60} and d_{20}/d_5 (where d_{90}, d_{60}, d_{20} and d_5 are the sieve sizes for which 90%, 60%, 20% and 5% respectively of the weighed soil is finer). More recently, Chang and Zhang (2013) proposed three categories of soil erodibility from the comparison of three criteria. They distinguished widely graded and gap graded soils. Chang and Zhang defined P as the mass fraction of particles finer than 0.063mm, and in the case of gap graded soil the gap ratio as: $G_r = d_{max}/d_{min}$ (d_{max} and d_{min}: maximal and minimal particle sizes characterizing the gap in the grading curve). For P less than 10%, the authors assumed that the stability is correctly assessed using the criterion $G_r < 3$. For P higher than 35%, the gap graded soil is reputed stable, and with P in the range 10% to 35% the soil is stable if $G_r < 0.3P$. According to Chang and Zhang, their method is only

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applicable to low plasticity soils. From the comparisons of criteria realized by Li and Fannin (2008), Wan and Fell (2008) and Chang and Zhang (2013), Marot et al. (2016) identified the less conservative criteria for potential susceptibility to suffusion for cohesionless soils and clayey sands.

However for the same granular distribution, suffusion tests performed on different mixtures of low percentages of kaolin with aggregates showed that angularity of coarse fraction grains contributes to increase the suffusion resistance (Marot et al. 2012). Thus the shape of grains also plays an important role on suffusion susceptibility. Marot et al. (2012) used three methods for characterizing grain shape: digital picture analyses, direct shear tests and by gravitating flows with a sand angulometer. Whatever indicator was considered for grain shape characterizing, the same relative classification of the tested aggregates was obtained. However, the measurement of internal friction angle under the same density index I_d ($I_d = [e_{max} - e]/[e_{max} - e_{min}]$ where e_{max}, e_{min} are the maximum and minimum values respectively of the void ratio e) appears to be the more appropriated to characterize the influence of the grain shape on the process of suffusion.

In addition with material susceptibility, the stress condition also can influence the suffusion susceptibility. Several tests performed in oedometric conditions on unstable soils showed that a rise in the effective stress causes an increase of the soils' resistance to suffusion (Moffat and Fannin 2006). In the same manner, when tests were carried out under isotropic confinement (Bendahmane et al. 2008), the increase in the confinement pressure and the subsequence local increase of soil density (i.e. smaller size of constrictions between coarse grains) allowed a decrease in the suffusion rate.

The third condition for suffusion initiation is related to the hydraulic loading on the grains which is often described by three distinct parameters: the hydraulic gradient, the hydraulic shear stress and the pore velocity. The critical values of these three quantities can then be used to characterize the suffusion initiation (Skempton and Brogan 1994; Moffat and Fannin 2006; Perzlmaier 2007 among others).

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However a fraction of the detached particles can re-settle or be filtered at the bulk of the porous network (Reddi et al. 2000; Bendahmane et al. 2008; Marot et al. 2009; 2011a; Nguyen et al. 2012; Luo et al. 2013). The processes of detachment, transport and filtration of fine particles are thus inseparable. These processes can eventually induce local clogging, accompanied by variations of fluid velocity and interstitial pressure. Therefore, variations of both seepage flow and pressure gradient have to be taken into account to evaluate the hydraulic loading during suffusion development.

Suffusion susceptibility classification

Further to results of concentrated leak erosion tests, Marot et al. (2011b) proposed a new analysis based on the energy expended by the seepage flow which is a function of both the flow rate and the pressure gradient. Three assumptions were used: the fluid temperature is assumed constant, the system is considered as adiabatic and only a steady state is considered. The energy conservation equation permits to express the total flow power as the summation of the power transferred from the fluid to the solid particles and the power dissipated by viscous stresses in the bulk. As the transfer appears negligible in suffusion case (Sibille et al. 2015), the authors suggest to characterize the fluid loading from the total flow power, P_{flow} [in Watt] which is expressed by:

[1] $$P_{flow} = \left(\gamma_w \Delta z + \Delta P\right) Q$$

where γ_w [N/m^3] is the specific weight of water; $\Delta z = z_A - z_B$ [m], with z_A and z_B the vertical coordinates of the upstream section A of the soil volume and the downstream one B; $\Delta P = P_A - P_B$ [N/m^2] is the pressure drop between the sections A and B respectively and Q [m^3/s] is the volumetric water flow rate.

$\Delta z > 0$ if the flow is in downward direction, $\Delta z < 0$ if the flow is upward and the erosion power is equal to $Q\,\Delta P$ if the flow is horizontal.

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The expended energy E_{flow} [in Joule] is the time integration of the instantaneous power dissipated by the water seepage for the test duration [s]. For the same duration the cumulative loss dry mass is determined and the erosion resistance index is expressed by:

[2] $$I_\alpha = -\log\left(\frac{\text{Cumulative loss dry mass}}{E_{flow}}\right)$$

Depending on the values of I_α [–] index, Marot et al. (2016) proposed six categories of suffusion susceptibility from Highly Erodible to Highly Resistant (corresponding susceptibility categories: Highly Erodible for $I_\alpha < 2$; Erodible for $2 \leq I_\alpha < 3$; Moderately Erodible for $3 \leq I_\alpha < 4$; Moderately Resistant for $4 \leq I_\alpha < 5$; Resistant for $5 \leq I_\alpha < 6$; and Highly Resistant for $I_\alpha \geq 6$).

Experimental investigation

Specific device for erodibility characterization

A triaxial erodimeter was designed to apply seepage flow on intact soil samples. A detailed description of the device was reported by Bedahmane et al. (2008) and a brief summary is provided hereafter.

As shown in Fig. 1, the used testing apparatus comprises a modified triaxial cell, a water supply system, a soil collection system, and a water collection system. The modified triaxial cell permits to saturate the sample in upward direction and to force fluid through the sample in downward direction during the erosion phase. The seepage flow is applied in downward direction in order to catch the eroded particles more easily. A pressure sensor is connected between the top and bottom of specimen to measure the pressure drop between upstream and downstream. The system to generate seepage flow under constant hydraulic gradient comprises an upstream water tank. The system to generate seepage flow in flow-rate-controlled conditions comprises a gear pump connected to a pressure sensor at its outlet. For both types of hydraulic loading, the fluid circulates into the top cap which contains a layer of glass beads to diffuse the fluid uniformly on the specimen top surface. The sample is supported by a

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lower grid where different wire meshes can be placed in order to take into account the effect of pore opening size on internal erosion (Marot et al. 2009). For this study, the opening size of the selected mesh screen is 4 mm in order to allow the migration of all grains and in order to reproduce in-situ earth structures without filter, as a dike for example. The funnel – shaped draining system is connected to the effluent tank by a glass pipe. In the case of silt or clay suffusion, a multi-channel optical sensor is placed around the glass pipe (Marot et al. 2011a). Thanks to a previous calibration, the optical sensor allows measuring the silt or clay concentration within the effluent which is expressed as the ratio of the mass of fine particles to water mass within the fluid with a maximum relative error of 5% (Marot et al 2011a). The time integration of the fine particle concentration gives the cumulative eroded dry mass for the corresponding duration (Bendahmane et al. 2008) with a maximum relative error of 7%. Moreover, the detection of sand grains in the effluent is assessed thanks to the comparison of the voltages of each LED of the optical sensor (Marot et al. 2011a). For a high value of silt concentration within effluent or when effluent contents sand grains, the mass solid measurement can be performed by continuous weighing as mass accuracy of a few milligrams is sufficient. The effluent tank is equipped with an overflow outlet (to control the downstream hydraulic head) and a rotating sampling system containing 8 beakers for the sampling of loss particles during saturation phase and eroded particles carried away with the effluent. At the overflow outlet of the effluent tank, water falls in a beaker that is continuously weighed in order to determine injected flow rate.

Testing materials

With the objective to obtain a large range of suffusion susceptibility, eighteen non plastic or low plasticity soils are selected for their different grain size distributions and different grain shapes. Seven soils come from existing earth structures: soils named DR-A, DR-B, DR-C and 3 come from different French dikes. Two natural soils from a French dike were sieved with two different maximum diameters

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which is 5 mm and 10 mm for soils CH-5 and CH-10, respectively. Soil named CD is a till from Canada which is used for a core of a dam currently under construction. Eleven soils were created by mixing different non-plastic soils: seven soils (1, 6, B, C, G3-11, G3-13, G3-14) are composed by mixtures of sand S1 and gravel G3 (marketed by Sablière Palvadeau, France), soil 4 is created by the mixture of Fontainebleau sand and gravel G3, soil 5 is the mixture of sand S1, silt Limon Jossigny and gravel G3. Finally, two soils (R1 and R2) are composed by mixtures of sand S1 ($d < 0.63$mm) and DR ($d > 0.63$mm). A laser diffraction particle-size analyzer was used to measure the grain size distribution of tested soils with demineralized water and without deflocculating agent (see Fig. 2a and 2b).

Physical parameters

A set of fourteen physical parameters is measured. The selection of these parameters were realized according to the aforementioned identification of predominant parameters. Moreover, the goal of this study is to estimate soil erodibility by physical parameters that may be easily measured on site or on disturbed samples. In accordance with aforementioned grain size distribution based criteria, measured parameters include the uniformity coefficient C_u, the gap ratio G_r and the percentage P finer than 0.063 mm (see Table 1). The grain size analysis is also completed by d_5, d_{15}, d_{20}, d_{50}, d_{60}, d_{90} (diameters of the 5%, 15%, 20%, 50%, 60%, 90% mass passing, respectively). For widely graded soils, the fine fraction can be identified within the granular distribution by the minimum value of Kenney and Lau (1985)'s ratio min(H/F) and the corresponding fine percentage is named Finer KL. With the objective to take into account the influence of grain shape, internal friction angle φ of mixtures was determined thanks to a direct shear stress device (Marot et al. 2012). The shear tests are carried out on dry aggregates, with density index I_d near to 1. The testing method used is described by standard NF P94-071-1 (Association Française de Normalisation 1994). For low plasticity soils, the percentage of clay but also the mineralogy and chemical composition of clay give soils a different water sensitivity and a

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different sensitivity to erosion processes (Haghighi 2012). In consequence the blue methylene value V_{BS} is also measured because it permits quite easily and rapidly to highlight the water sensitivity of tested soils.

Furthermore, based on the compared criterion of Marot et al. (2016), the potential susceptibility classification is also added in Table 1. Now, according to this criterion, thirteen soils appear Unstable (1, 4, 5, 6, B, C, DR-B, DR-C, G3-11, G3-13, G3-14, R1, R2) and five soils appear Stable (DR-A, 3, CH-5, CH-10, CD).

Test procedure and testing program

Thirty one tests were carried out on samples in oedometric conditions, i.e. with no lateral displacements. As recommended by Kenney and Lau (1985), in order to reduce preferential flow, a membrane is placed between specimen and metal mold. Six specimens (50 mm in diameter and 100 mm in height, see Table 2) are produced by air pluviation directly into a membrane which is fixed by the metal mold and compacted until obtaining the target specimen volume in order to reach the target value of dry unit weight. Twenty five specimens are prepared using a single layer semi-static compaction technique (Camapum De Carvalho et al. 1987) with a 50 mm diameter and 50 mm high mold. Each specimen is wrapped in a membrane, and then closed inside the metal mold. After this step, carbon dioxide is upwardly injected, followed by the upwardly saturation phase which requires approximately 24 h. Finally, all specimens are subjected to a seepage flow in downward direction with deaerated and demineralized water. Two types of hydraulic loading are used: multi-staged hydraulic gradient condition, which consists of increasing the hydraulic gradient by steps and flow rate controlled condition. For soils 3 and R2, two different values of initial dry unit weight are used. Table 2 indicates the initial dry unit weight of specimens, the values of applied hydraulic gradient or injected flow rate

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for the tested specimens. The repeatability of tests was verified by performing two tests under identical conditions: DR-C1 and DR-C2.

Suffusion test results

Thanks to the measurements of seepage flow and pressure gradient, and based on Darcy's law, it is possible to compute the hydraulic conductivity.

The initial and final values of hydraulic conductivity measured during each test are detailed in Table 2. The repeatability of the seepage test can be validated by comparing the initial and final values of hydraulic conductivity for tests DR-C1 and DR-C2 which are in good agreement. Figure 3 shows the time evolution of hydraulic gradient for soil B according to the different types of hydraulic loading and Figure 4 shows the corresponding evolutions of hydraulic conductivity. When the applied hydraulic gradient is increased by steps (test B-i1), the hydraulic conductivity first decreases. The second phase of hydraulic conductivity evolution is characterized by a rapid increase (by a factor of 18 for this test). Finally the hydraulic conductivity reaches a constant value. Fig. 4 shows also the slow decrease with the time of the hydraulic conductivity which is measured under constant flow rate controlled test (test B-q1). Thus some variation in the hydraulic loading appears necessary in order to produce the second increasing phase of the hydraulic conductivity.

The comparison of time evolution of hydraulic conductivity with time evolution of erosion rate can provide further information to improve the understanding of suffusion process. The rate of erosion is expressed per unit cross section by:

[3] $$\dot{m} = \frac{m_{eroded}(\Delta t)}{s\,\Delta t}$$

where $\dot{m}$: rate of erosion [in $g.m^{-2}.s^{-1}$], $m_{eroded}(\Delta t)$ is the mass of eroded particles for the duration Δt [s], and s [m^2] is the cross section of the specimen.

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The rate of erosion versus time is plotted in Fig. 5 for tests B-q1 and B-i1. The decrease of hydraulic conductivity is systematically accompanied by a decrease of erosion rate, which suggests that some detached particles can be filtered within the soil itself. This filtration may induce a clogging of several pores and then a decrease of the hydraulic conductivity. In multi staged hydraulic gradient condition (test B-i1), a rough increase of the erosion rate occurs simultaneously with the increase of the hydraulic conductivity, confirming the assumption of a clogging firstly restricting the water flow and then blown by the seepage flow itself. Thus the predominant process during this second phase seems to be the detachment and transport of solid particles. Finally hydraulic conductivity tends to stabilize while the erosion rate decreases. This third phase could be explained by the presence of preferential flows created by the erosion process leading to a steady state which is pointed out by black spots on Fig. 4 and Fig. 5. Therefore, these results show that the history of the hydraulic loading has a significant influence on the hydraulic behavior of the specimens and on the development of the suffusion. Moreover the erosion phenomenon of suffusion appears as a combination of three processes: detachment, transport and possible filtration of finer fraction.

For characterizing the erosion susceptibility, the cumulative expended energy per unit volume and the cumulative loss dry mass per unit volume (which includes the mass lost during saturation phase and the eroded mass) are determined at the end of the test which is defined by the steady state. The results of all tested specimens are shown on Figure 6 with the corresponding susceptibility categories. Table 3 details test duration and corresponding values of: cumulative loss dry mass per unit volume, expended energy per unit volume and erosion resistance index, computed by eq. [2] for all realized tests. The accuracy of the erosion resistance index measurement is evaluated to ± 0.02. Moreover the repeatability tests DR-C1 and DR-C2 lead to an erosion resistance index equal to 2.5 and 2.7, respectively. This discrepancy of 0.2 is due to the measurement accuracy but also to the accuracy related to the specimen

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creation. Thus this value is used to define the borders of susceptibility $I_a \pm 0.1$ between two classifications which are indicated in Table 3. It is worth noting that for a given soil and a given initial dry unit weight (see soils B and R2 in Table 3), the corresponding value of erosion resistance index, and then the corresponding suffusion susceptibility classification can be determined with accuracy for different hydraulic loadings. Even if the tests were performed with different specimen heights (soils 4, 5 and 6) the erosion resistance index values are in good agreement. According to the suffusion susceptibility classification, R1-90b, CH-10 and CD are Resistant, three specimens (DR-A, 3-T-1, CH-5) are Moderately Resistant, G3-14 is between Moderately Resistant and Moderately Erodible, sixteen specimens are Moderately Erodible (1-T-1, 4-T-1, 4-T-2, 5-T-1, 5-T-2, 6-T-1, 6-T-3, B-q1, B-q2, B-i1, B-i2, B-90a, B-90b, G3-13, R2-97b, R2-97d), two are between Moderately Erodible and Erodible (DR-B and R2-90b), six are Erodible (C, DR-C1, DR-C2, G3-11, 3-T-2, R2-90a).

Discussions

Comparison between suffusion susceptibility classification and grain size based criteria

From the criterion of US Army Corps of Engineers (1953), it is not possible to estimate the potential susceptibility classification because suffusion test results showed that soils 1, 4 and 6 are Moderately Erodible, whereas their values of C_u are lower than 20. Further a higher value than 20 can be associated with Moderately Resistant soil (soil 3). Thanks to the re-evaluation and the identification of less conservative grain size based criteria proposed by Marot et al. (2016), the potential susceptibility classification and the suffusion susceptibility classification are in general in agreement for most of tested soils. However a soil classified as Unstable by the potential susceptibility classification (Marot et al., 2016) can be either Moderately Resistant - Moderately Erodible (soil G3-14), Moderately Erodible (soil R2) or Erodible (soils C, DR-C, G3-11). In the same manner, Stable classification corresponds to suffusion susceptibility between Moderately Resistant (soils DR-A, CH5) and Resistant (soils CH10,

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CD). Moreover the gain size based classification of specimen R1-90b is Unstable and its suffusion susceptibility is Resistant. On the contrary, specimen 3-T-2 is classified Stable whereas its suffusion classification is Erodible. From the same soil but with a higher initial dry unit weight, specimen 3-T-1 is Moderately Resistant. Thus these results show the necessity for suffusion susceptibility estimation, to take into account physical parameters in addition with grain size distribution.

Principal Component Analysis

In conformity with the aforementioned identification of predominant parameters, the used physical parameters in this statistical analysis include: the dry unit weight of the soil γ_d, the internal friction angle φ and the blue methylene value V_{BS}. For the characterization of the grain size distribution, ten variables are used: the minimum value of ratio H/F min(H/F), gap ratio G_r, Finer KL, P, d_5, d_{15}, d_{20}, d_{50}, d_{60} and d_{90}. As the potential susceptibility classification based on the uniformity coefficient C_u is not consistent with suffusion test results, this parameter is not used for the statistical analysis. All aforementioned parameters are related to the soil properties, but suffusion process is a fluid solid interaction. Thus it seems to be interesting to complete the soil description by the initial hydraulic conductivity k_i, although its measurement is more binding than the measurement of the other parameters.

In the Principal Component Analysis, each parameter is represented in a factor space, assuming a linear correlation between the variables. The geometrical representation associates a vector to each parameter and the scalar product of two associated vectors is equal to the correlation coefficient of the two parameters. An automatic classification is used to define all variables according to the most useful factors. Figures 7a and 7b show the fourteen parameters and the erosion resistance index I_α in two first factor planes 1-2 and 2-3 respectively. From these Figures, it can be observed that no parameter is linearly correlated with erosion resistance index. However the following variables are close to each

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other on both two factor planes: d_{15} and d_{20}, and to a lesser extent d_{50} and d_{60}. It means that they are significantly positively correlated with each other. Gap ratio G_r and min(H/F) appear negatively correlated as they are on the opposite side of the center.

Multivariate analysis

By leading a multivariate analysis, a correlation with erosion resistance index and the fourteen aforementioned parameters is proposed:

$$[4] \quad I_\alpha = -13.57 + 0.43\gamma_d + 0.18\varphi - 0.02\text{Finer KL} + 0.49V_{BS} + 189.70k_i + 3.82\ \min(H/F)$$

$$+0.18P + 0.28G_r + 19.51d_5 + 1.06d_{15} - 0.84d_{20} + 0.81d_{50} - 0.98d_{60} - 0.10d_{90}$$

The obtained correlation coefficient between the prediction and the measurement is $R^2 = 0.94$ for a sample size N = 31. Figure 8 shows the erosion resistance index values, computed by eq. [4], versus the measured values.

Ten parameters (γ_d, φ, V_{BS}, G_r, k_i, min(H/F), P, d_5, d_{15}, d_{50}) contribute to eq. [4] with positive sign. On the contrary, the terms with Finer KL, d_{20} d_{60} and d_{90} are negative. Because of the coupling between several parameters, it is difficult to evaluate the contribution of each one.

From the Principal Component Analysis, a reduction of the number of physical parameters can be performed by eliminating d_{15}, d_{50} and min(H/F) which are close or opposite on both two factor planes to d_{20}, d_{60} and G_r respectively (see Fig. 7(a) and 7(b)). Moreover, as the goal of this study is to optimize the experimental campaign by using only quite easily measured parameters, the initial hydraulic conductivity is not used. Finally, thanks to the values of gap ratio, it is possible to distinguish the gap-graded soils (characterized by $G_r > 1$) from the widely-graded soils. A new multivariable analysis can permit to build a correlation with the corresponding reduced number of physical parameters. For gap-graded soils, the statistical analysis on twenty one specimens leads to the following expression:

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[5] $I_\alpha = -37.62+0.67\,\gamma_d + 0.64\,\varphi + 0.09\text{Finer KL}-0.03V_{BS} -1.43P +0.63G_r +0.76d_5 - 0.97d_{60}+0.61d_{90}$

$(R^2 = 0.88, N = 21)$

For Widely-graded soils, the new correlation is:

[6] $I\alpha = -26.34+0.43\gamma_d + 0.66\,\varphi - 0.16\text{Finer KL} + 1.15V_{BS} +0.37P +6.82d_5 -1.26\,d_{60}$

$(R^2 = 0.99, N = 10)$

The erosion resistance index values computed by eq. [5] and [6] are plotted versus the measured values in Figures 9a and 9b respectively. If we consider the values of parameters and associated factors in eq. [5] and [6], it is worth noting the key contribution of the dry unit weight and the internal friction angle. This result is consistent with the coupled influence of grain size distribution, grain shape and porosity on both aforementioned parameters and also on suffusion susceptibility.

Conclusions

A specific erodimeter is used to study the suffusion susceptibility of thirty one specimens of eighteen different soils. Tests realized under different hydraulic loading histories highlight the complexity of suffusion which appears as the result of coupling effects of three processes: detachment, transport and filtration. The interpretation of such tests is based on the evaluation of the hydraulic loading thanks to the expended energy on one hand, and the cumulative loss dry mass for the soil response on the other hand. At the steady state, which corresponds to the invariability of the hydraulic conductivity and the decrease of erosion rate, the energy based method permits to determine the suffusion susceptibility and the erosion resistance index is computed.

The fourteen following physical parameters were also measured: the dry unit weight of the soil γ_d, the internal friction angle φ, the blue methylene value V_{BS}, the minimum value of ratio H/F, gap ratio G_r, P, Finer KL, initial hydraulic conductivity k_i, and diameters d_5, d_{15}, d_{20}, d_{50}, d_{60} and d_{90}. A statistical

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analysis is performed and shows that no parameter is linearly correlated with the erosion resistance index.

Now by focusing on easily measured parameters and by distinguishing the gap-graded soils and widely-graded soils, the multivariate statistical analysis leads to an expression of the erosion resistance index for gap graded soils with respect to nine physical parameters: γ_d, φ, Finer KL, V_{BS}, P, gap ratio G_r, d_5, d_{60} and d_{90}. For widely graded soils, a new correlation erosion resistance index with seven parameters: the dry unit weight of the soil γ_d, φ, Finer KL, V_{BS}, P, d_5 and d_{60}. Thus, this method allows an optimization of any experimental campaign of suffusion susceptibility characterization by reducing the number of variables for the description of this susceptibility.

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List of symbols

C_u uniformity coefficient

d_5 diameter of 5% mass passing

d_{15} diameter of 15% mass passing

d_{20} diameter of 20% mass passing

d_{50} diameter of 50% mass passing

d_{60} diameter of 60% mass passing

d_{90} diameter of 90% mass passing

e void ratio

e_{max} maximum value of void ratio

e_{min} minimum value of void ratio

E_{flow} expended energy by seepage flow

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Finer KL percentage of finer based on Kenney and Lau'criteria

G_r gap ratio

I_d density index

I_α erosion resistance index

k_i initial hydraulic conductivity

$\dot{m}$ rate of eroded dry mass per unit cross section

min(H/F) minimum value of ratio H and F based on Kenney and Lau'criteria

N number of samples

P percentage of particle smaller than 0.063mm

P_{flow} total flow power

Q volumetric flow rate

s cross section of specimen

S stable

R^2 correlation coefficient

U unstable

V_{BS} blue methylene value

γ_d dry unit weight

φ internal friction angle

γ_w specific weight of water

ΔP pressure drop

Δt duration

Δz difference of vertical coordinates

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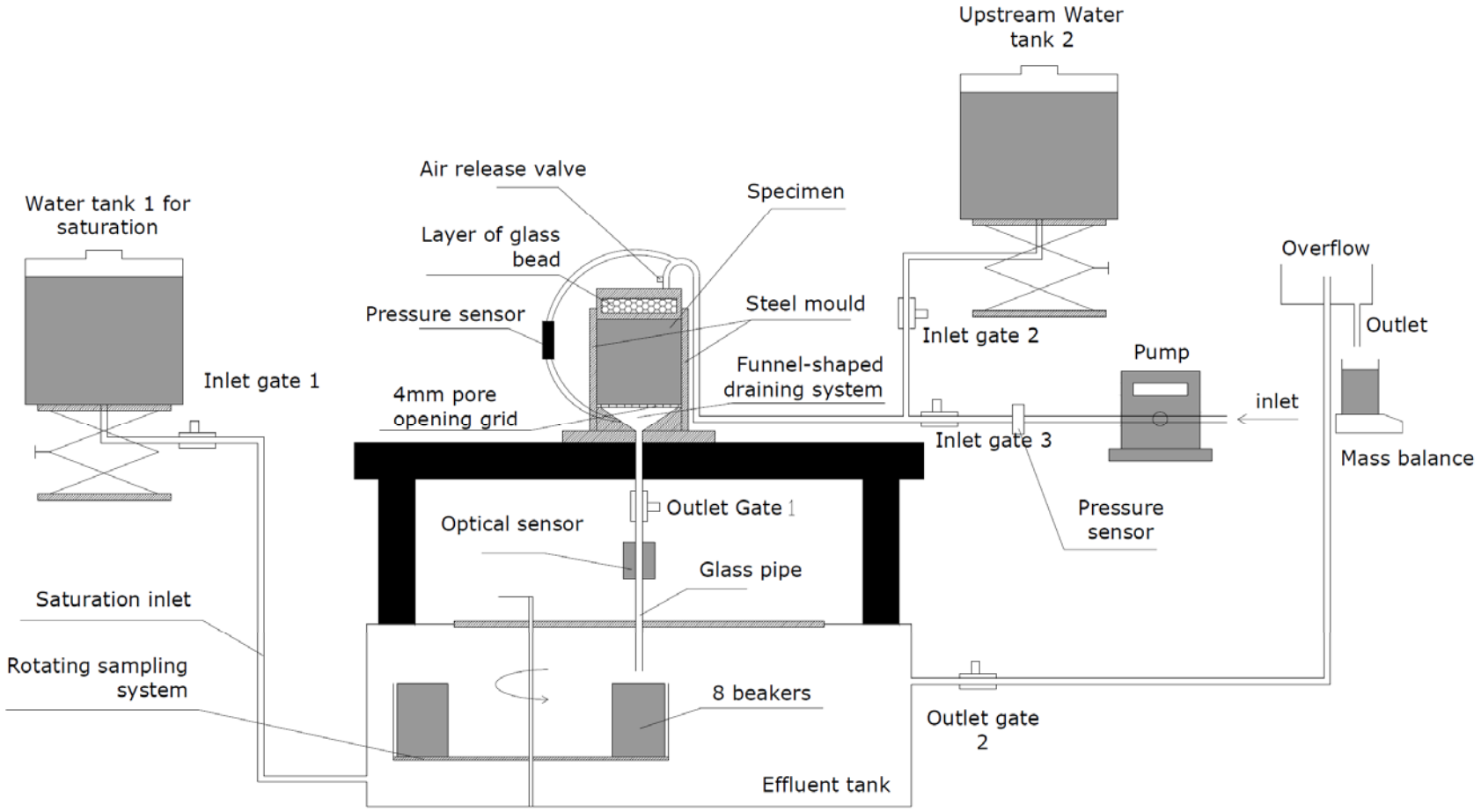
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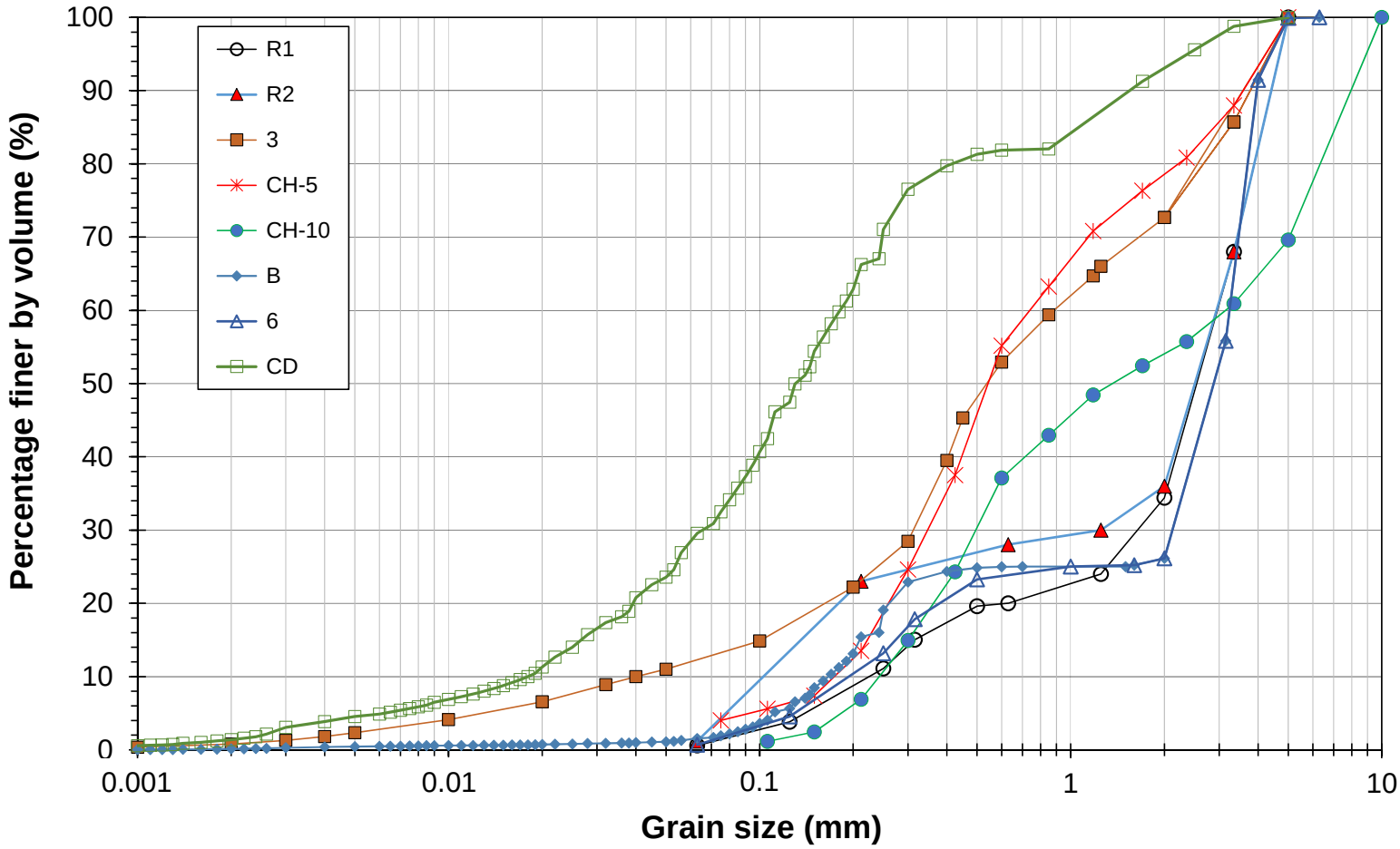
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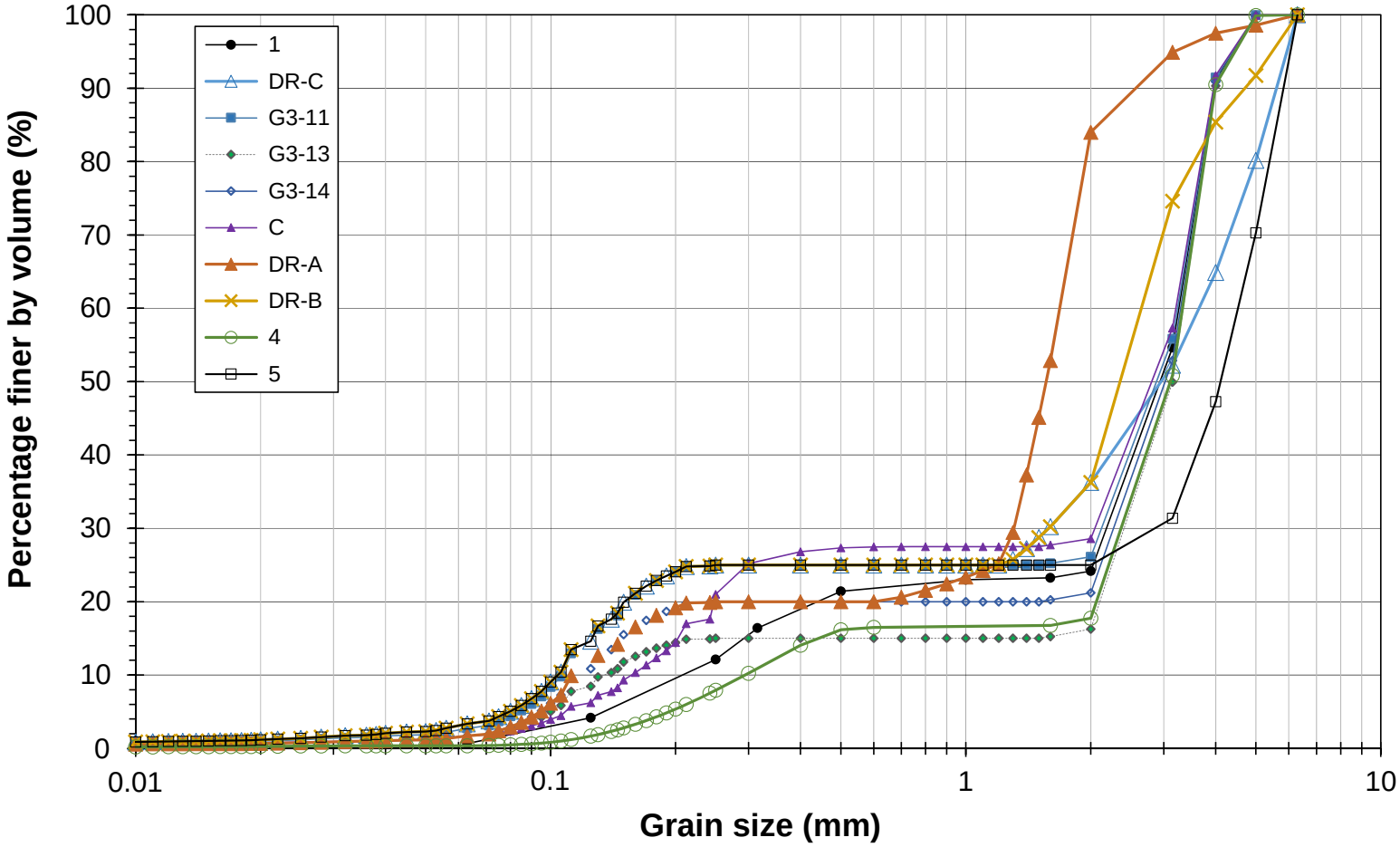
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(a)



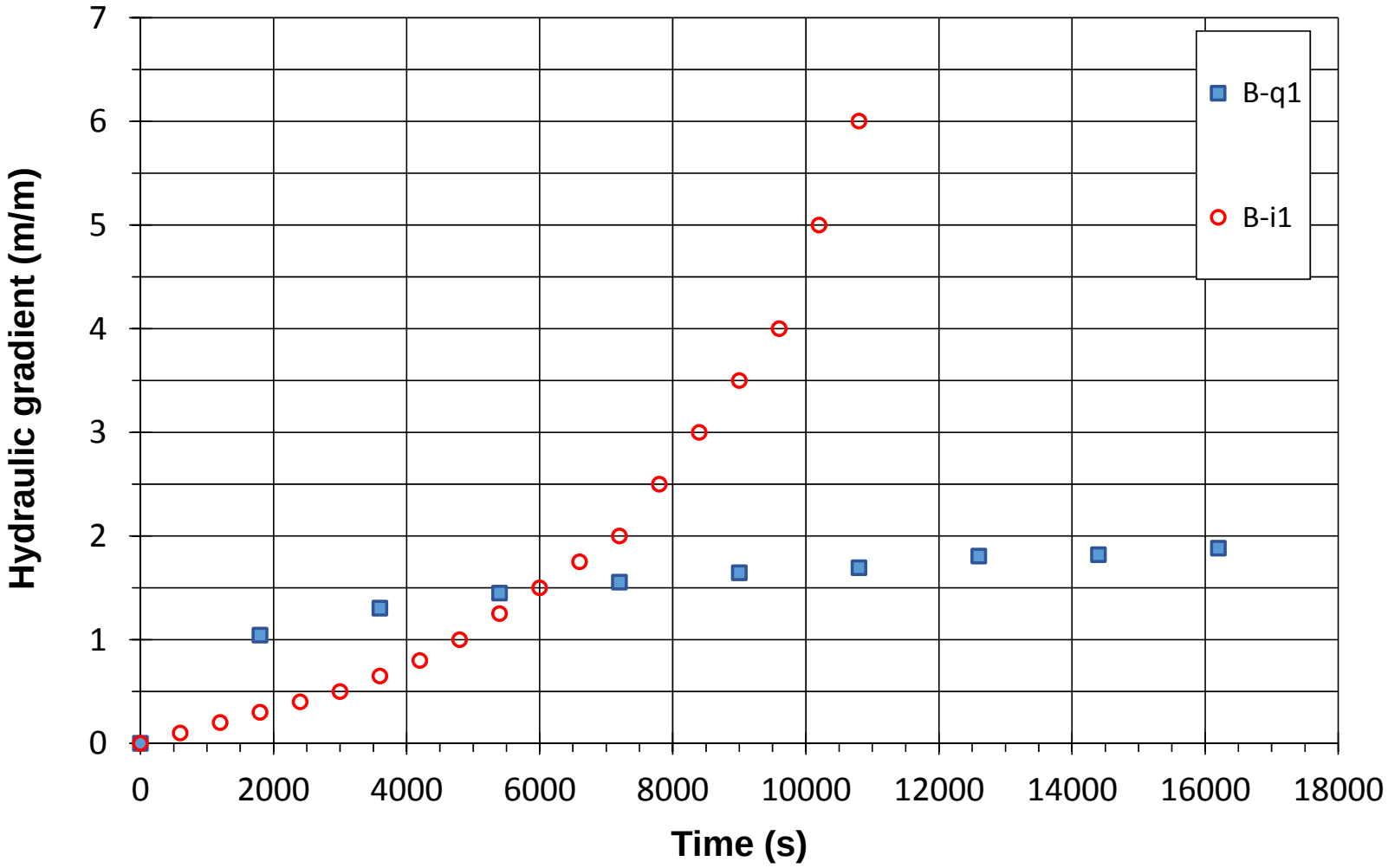
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(b)



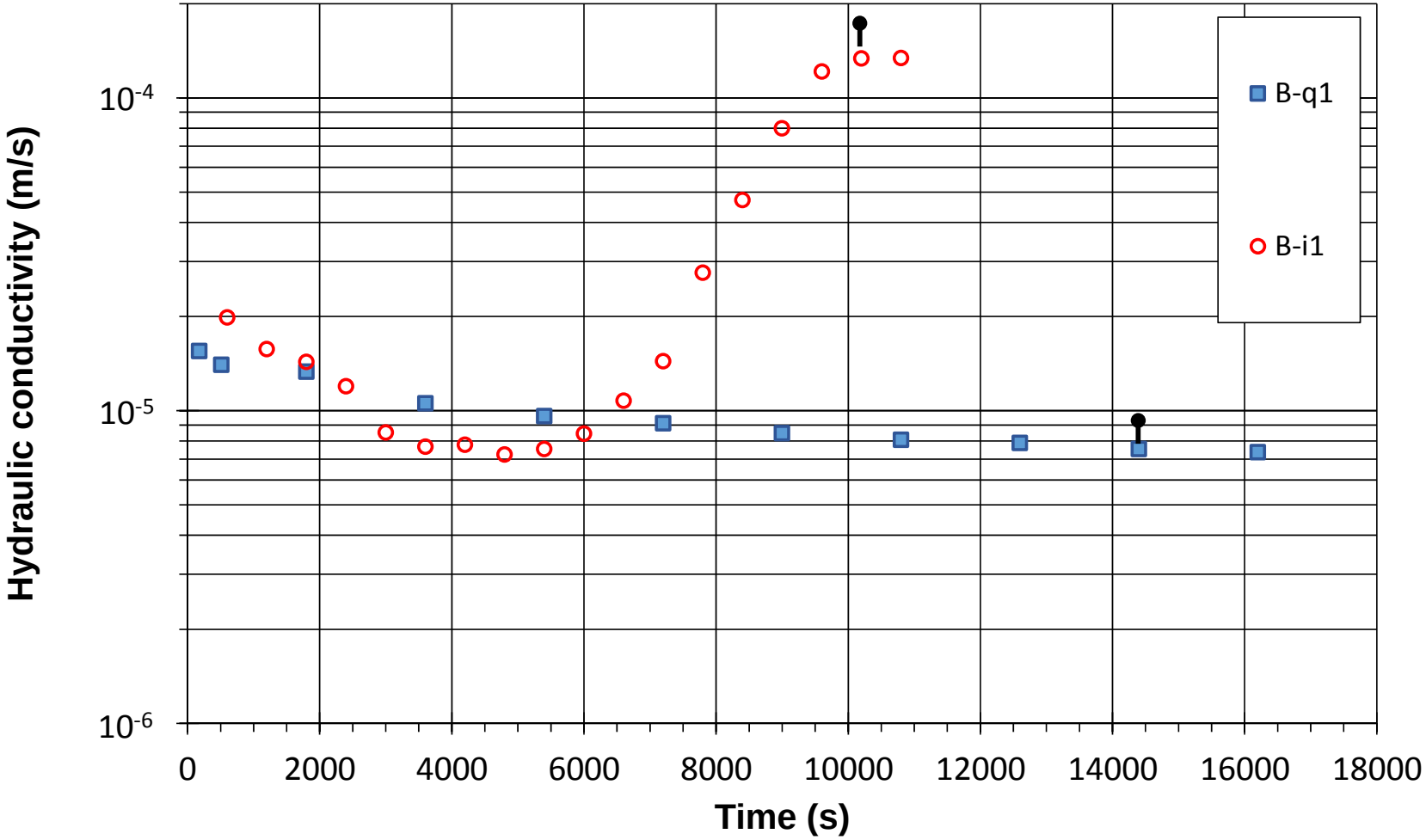
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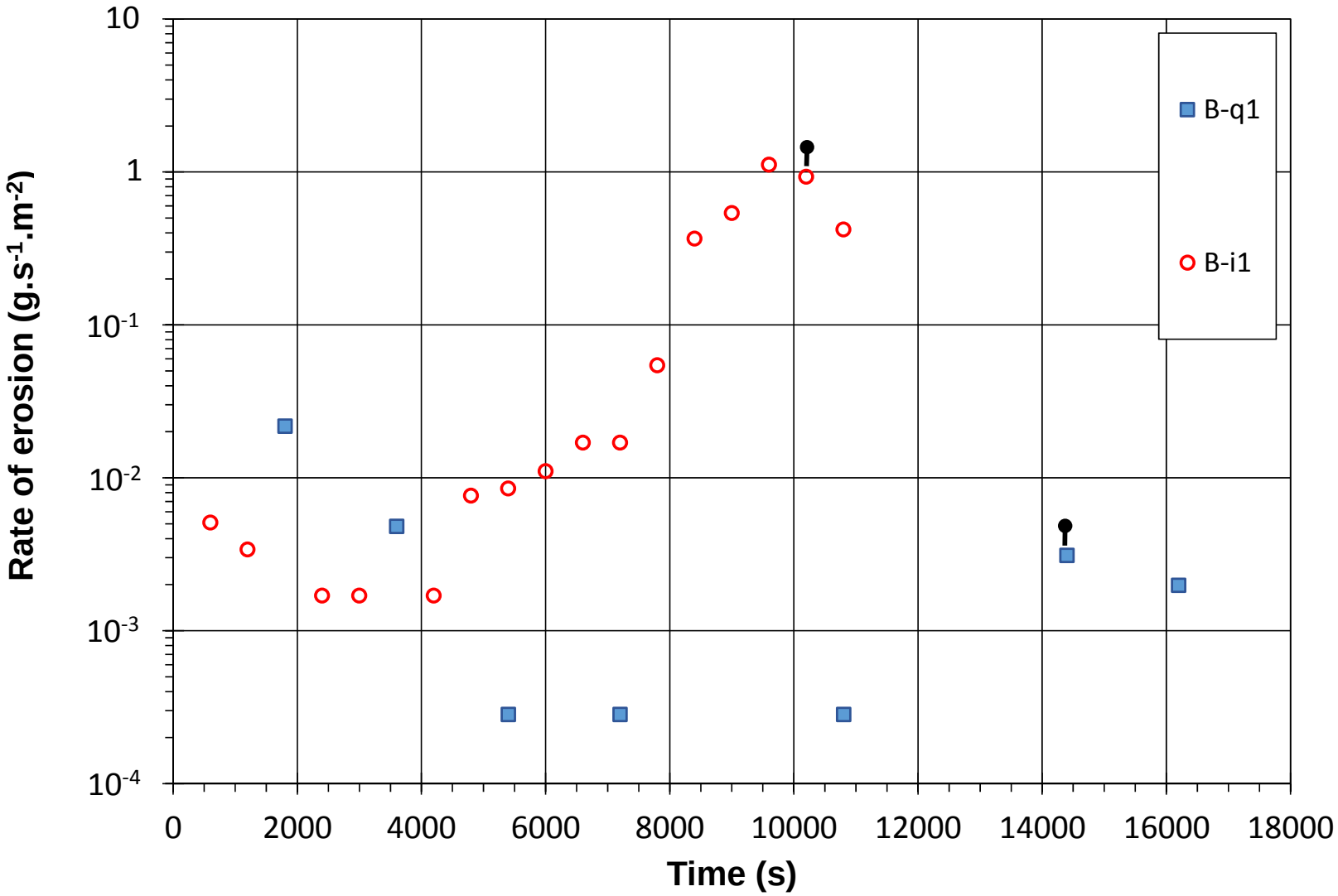
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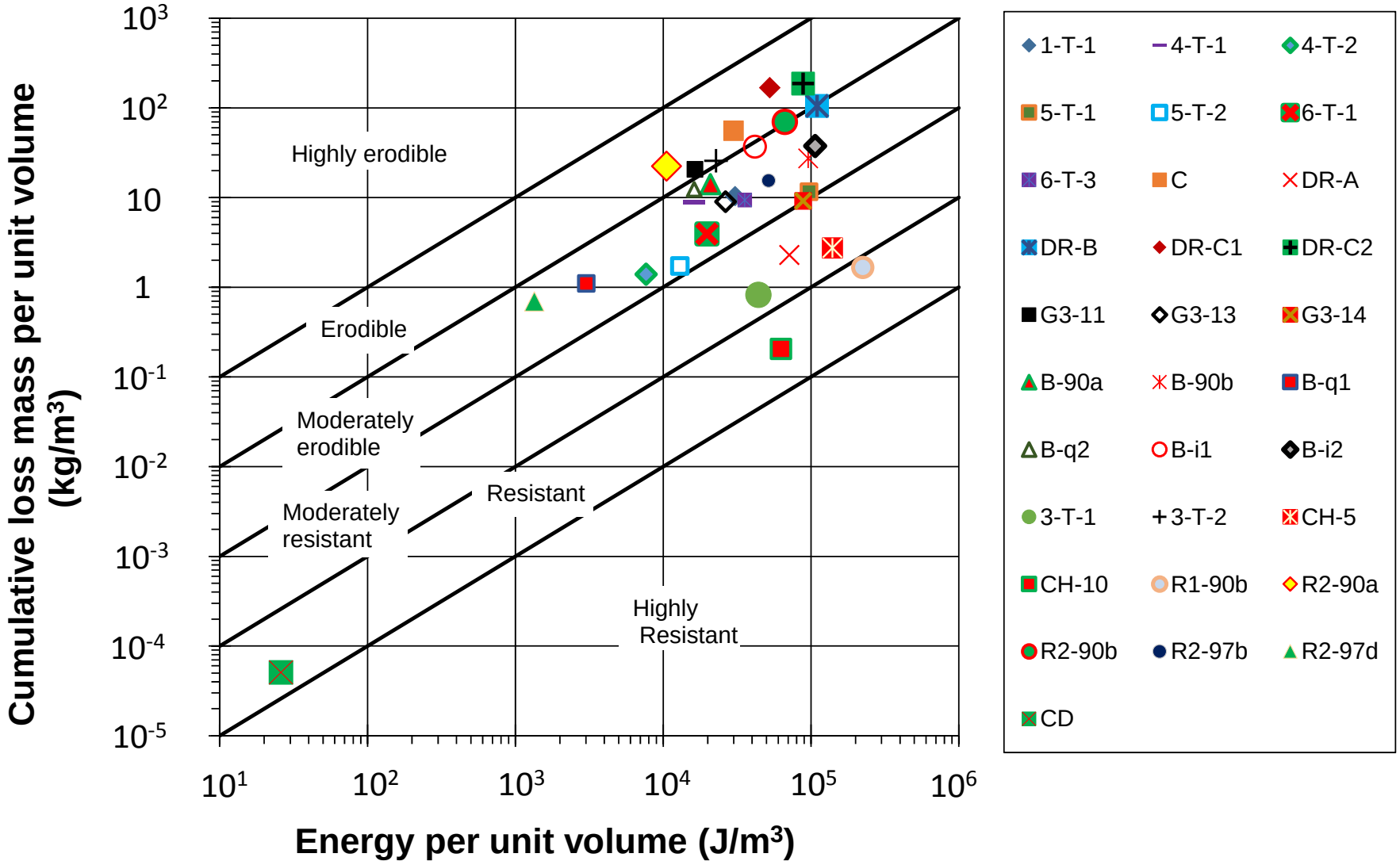
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Fig.5. Time series of erosion rate (tests B-q1, i1). Black spots show time of steady state.



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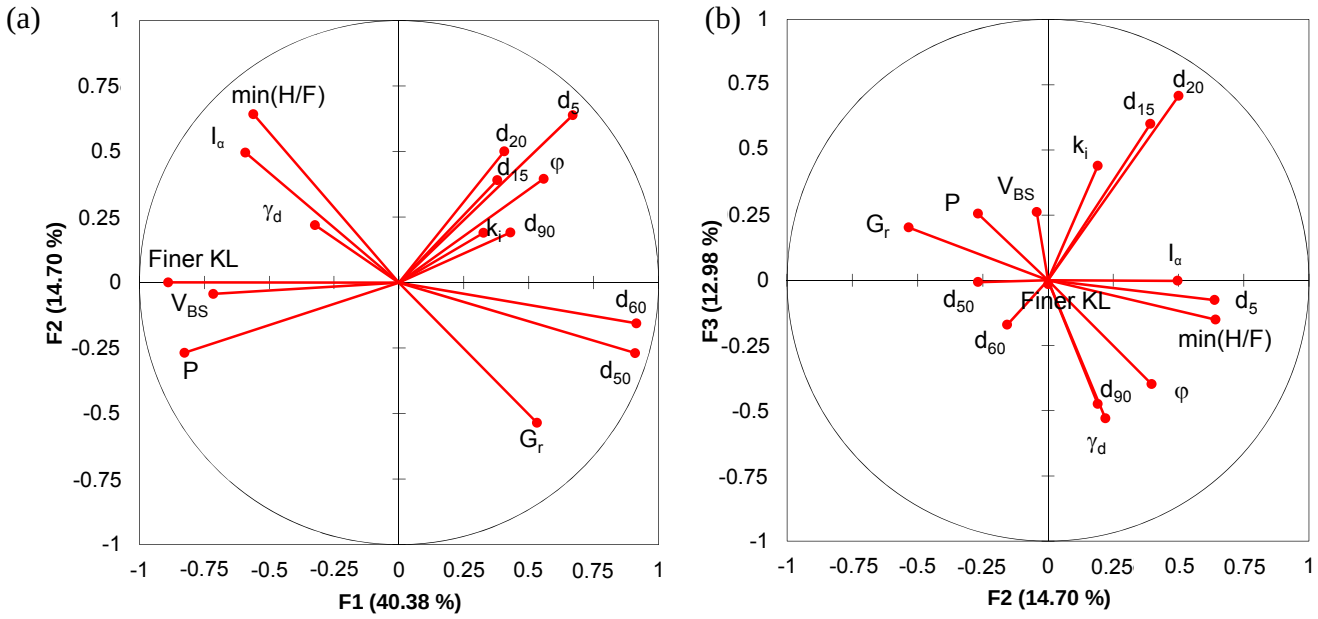
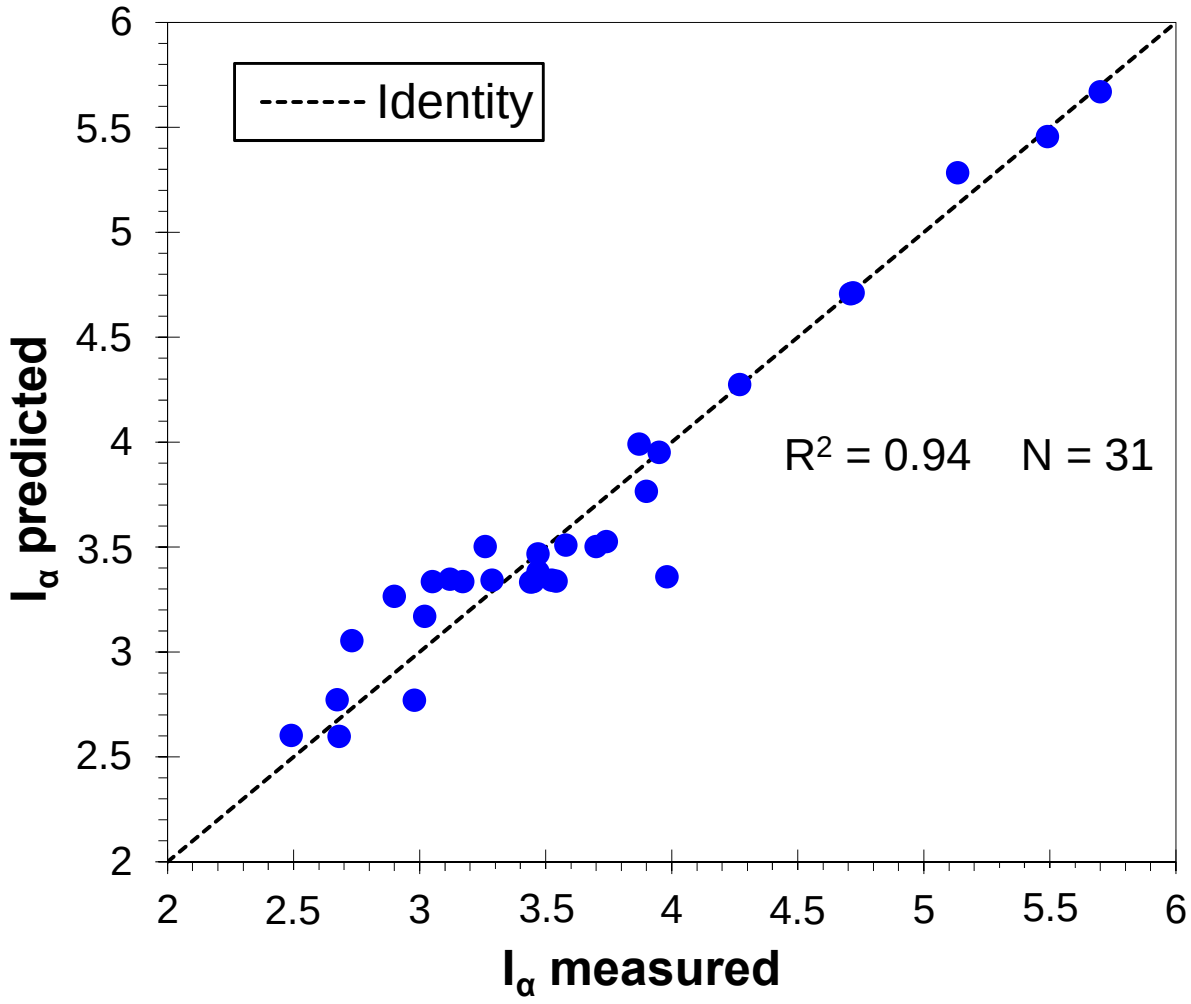


Figure 7. Representation of variables in (a) factor plane 1-2 and (b) factor plane 2-3.

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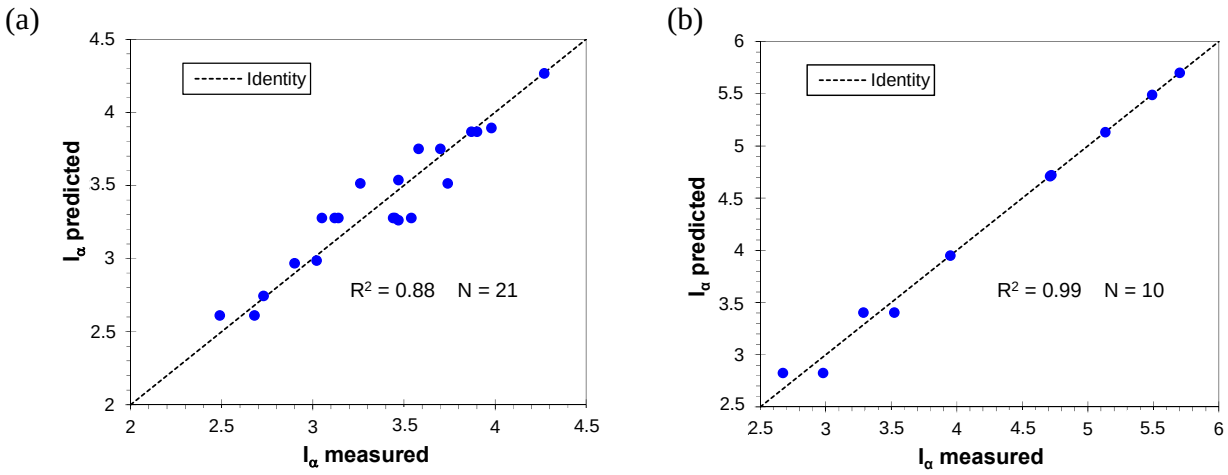


Figure 9. Erosion resistance index, predicted values versus measured values for (a) gap graded soils (with 9 parameters) and (b) widely graded soils (with 7 parameters).

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Table 1. Properties and potential susceptibility of tested soils.

Soils	φ (°)	Finer KL (%)	V_{BS} (g/100g)	min (H/F) (-)	d_5 (mm)	d_{15} (mm)	d_{20} (mm)	d_{50} (mm)	d_{60} (mm)	d_{90} (mm)	C_u (-)	G_r (-)	P (%)	Potential Susceptibility (Marot et al' Criterion)
1	44	23	0.1	0.125	0.14	0.294	0.45	2.97	3.27	3.97	14.86	1.6	0.64	U
4	44	16.5	0.07	0.094	0.193	0.445	2.08	3.12	3.347	3.99	11.42	2.67	0.36	U
5	44	25	0.15	0	0.14	0.126	0.45	2.97	3.27	3.97	43.75	8	3.34	U
6	44	25	0.11	0.12	0.193	0.276	2.08	3.12	3.347	3.99	15.88	1.6	0.7	U
B	44	25	0.163	0.035	0.08	0.21	0.15	4.12	4.55	5.86	19.58	2.5	1.6	U
C	43	27.5	0.179	0.034	0.13	0.198	0.389	2.92	3.25	3.97	20.53	2.5	1.7	U
DR-A	45	20	0.13	0.109	0.094	0.148	0.25	1.563	1.692	2.633	14.91	2.4	1.7	S
DR-B	45	25	0.163	0	0.08	0.126	0.151	2.412	2.712	4.727	26.03	4.8	3.3	U
DR-C	45	25	0.163	0	0.08	0.126	0.151	2.99	3.671	5.645	35.25	4.8	3.3	U
G3-11	44	25	0.163	0	0.084	0.127	0.153	2.924	3.25	3.965	30.53	6	2.7	U
G3-13	44	15	0.098	0	0.1	1.5	2.127	3.15	3.362	3.993	25.04	6	1.6	U
G3-14	44	20	0.13	0	0.094	0.145	0.25	3.046	3.309	3.98	29.17	6	1.7	U
3	40	51.84	1.00	0.452	0.014	0.102	0.17	0.542	0.889	3.845	22.23	1	12.12	S
R1	44	15.26	0.11	0.593	0.145	0.315	0.63	2.627	3.029	4.483	13.17	1	0.59	U
R2	43	25.04	0.11	0.195	0.094	0.157	0.263	2.59	3.013	4.484	24.50	1	1.2	U
CH-5	49	60.01	0.41	0.413	0.094	0.224	0.263	0.549	0.75	3.629	4.25	1	3	S
CH-10	49	40.61	0.291	0.443	0.186	0.301	0.368	1.381	3.178	8.354	12.92	1	1	S
CD	37	76.46	0.70	0.11	0.0062	0.026	0.039	0.135	0.182	1.461	10.11	1	29.57	S

Finer KL and min(H/F) are based on Kenney and Lau (1986)'s criterion, F: mass percentage of the grains lowers than d, H: mass percentage of the grains between d and 4d. V_{BS}: the blue methylene value. φ: the internal friction angle. G_r: gap ratio; widely graded soils Gr =1; P: percentage of particle smaller than 0.063mm, G_r and P are based on Chang and Zhang (2013)'s criterion. U: Unstable, S: Stable, C_u: uniformity coefficient

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Table 2. Properties of specimens.

Soils	Tested specimens	Specimen height (mm)	Dry unit weight (kN/m^3)	Applied hydraulic gradient i (m/m)	Injected flow (ml/min)	Initial Hydraulic conductivity 10^{-5} (m/s)	Final hydraulic conductivity 10^{-5} (m/s)
1	1-T-1	50	16.43	0.4-3		3.2	7.0
4	4-T-1	50	16.13	0.1-1.5		61.5	21.3
	4-T-2	100	16.13	0.043-0.705		55.7	140.0
5	5-T-1	50	17	0.4-4		4.1	10.5
	5-T-2	100	17	0.012-0.81		87.0	163.0
6	6-T-1	50	17	0.094-7.50		63.3	0.3
	6-T-3	100	17	0.07-1.13		38.5	21.8
B	B-q1	50	17.39		1.60	1.6	0.8
	B-q2	50	17.39		12	8.2	6.7
	B-i1	50	17.39	0.1-6		2.0	13.3
	B-i2	50	17.39	1-10		2.6	7.2
	B-90a	50	17.39	0.38-2.04		2.0	13.3
	B-90b	50	17.39	0.77-1.98		3.9	53.2
C	C	50	17.39	0.1-7		1.4	6.5
DR-A	DR-A	50	17.87	0.1-16		2.2	1.4
DR-B	DR-B	50	16	0.1-7		4.1	11.6
DR-C	DR-C1	50	16	0.1-7		5.8	9.1
	DR-C2	50	16	0.1-7		2.8	14.3
G3-11	G3-11	50	16	0.1-5		7.3	3.5
G3-13	G3-13	50	16	0.1-6		14.5	14.9
G3-14	G3-14	50	16	0.1-8		4.6	13.8
3	3-T-1	100	17	0.106-4.65		6.0	7.2
	3-T-2	100	15.5	0.106-4.65		12.0	10.0
R1	R1-90b	50	17.39	1-11		3.4	9.7
R2	R2-90a	50	17.39	0.1-6		3.7	13.5
	R2-90b	50	17.39	1-8		2.4	15.7
	R2-97b	50	18.74	1-12		1.5	6.2
	R2-97d	50	18.74		1.25	1.3	1.0
CH-5	CH-5	50	16.54	0.1-14		9.0	6.1
CH-10	CH-10	50	18.90	0.1-16		8.0	1.6
CD	CD	100	19.14	0.06-6.02		3.0	0.1

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Table 3. Results of suffusion tests at steady state.

Specimens	Test duration (s)	Loss mass per unit volume (kg/m^3)	Expended energy per unit volume (J/m^3)	Erosion resistance index (I_α) [-]	Marot et al (2016) suffusion classification
1-T-1	10800	10.4	30612	3.5	ME
4-T1	9000	8.9	16249	3.3	ME
4-T-2	10200	1.4	7666	3.7	ME
5-T-1	10800	11.6	96933	3.9	ME
5-T-2	12600	1.7	12971	3.9	ME
6-T-1	15180	4.0	19745	3.7	ME
6-T-3	9420	9.4	35657	3.6	ME
B-q1	14400	1.1	3014	3.4	ME
B-q2	9000	12.4	16228	3.1	ME
B-i1	10200	37.0	41733	3.1	ME
B-i2	5400	37.6	106626	3.5	ME
B-90a	7200	14.1	20907	3.2	ME
B-90b	4200	27.4	95647	3.5	ME
C	11033.4	55.6	30000	2.7	E
DR-A	19877.6	2.3	71346	4.5	MR
DR-B	12020,3	105.0	109684	3.0	ME-E

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DR-C1	12582.6	168.5	52579	2.5	E
DR-C2	10820.2	186.6	88308	2.7	E
G3-11	10485.5	20.7	16340	2.9	E
G3-13	8418.8	9.0	26402	3.5	ME
G3-14	12020.7	9.2	88349	4.0	MR-ME
3-T-1	11867.6	0.8	44134	4.7	MR
3-T-2	11160	25.8	22737	3.0	E
R1-90b	6600	1.7	224525	5.1	R
R2-90a	9000	22.3	10536	2.7	E
R2-90b	3600	69.9	66522	3.0	ME-E
R2-97b	5400	15.5	51643	3.5	ME
R2-97d	12600	0.7	1344	3.3	ME
CH-5	16828	2.8	139755	4.7	MR
CH-10	14406.9	0.2	62931	5.5	R
CD	12600	$5\ 10^{-5}$	26	5.7	R

ME: Moderately Erodible, E: Erodible, MR: Moderately Resistant, R: Resistant